

STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	STATE DIST. NO.	COUNTY	SHEET NO.	TOTAL SHEETS

MIN. CONCRETE STRENGTH @ RELEASE	= 5500 PSI
MIN. CONCRETE STRENGTH @ 28 DAYS	= 8000 PSI
INITIAL PULL/STRAND	= 33,820 LBS
CROSS-SECTION AREA/STRAND	= 0.167 SQ.IN.

DESIGN DATA FOR 21" DEPTH ADJACENT BOX BEAM																	
SPAN LENGTH ϕ TO ϕ BEARING		30' -0"	32' -0"	34' -0"	36' -0"	38' -0"	40' -0"	42' -0"	44' -0"	46' -0"	48' -0"	50' -0"					
OVERALL LENGTH OF BEAM		31' -6"	33' -6"	35' -6"	37' -6"	39' -6"	41' -6"	43' -6"	45' -6"	47' -6"	49' -6"	51' -6"					
NO. OF 270 KSI, $\frac{1}{2}$ " ϕ LOW-RELAXATION STRANDS, AREA/STRAND = 0.167 SQ. IN.		10	10	10	12	12	14	14	14	16	16	16					
STRAND POSITION NUMBER	ROW 1	1,2,11,12	1,2,11,12	1,2,11,12	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14					
	ROW 2	15,16,25,26	15,16,25,26	15,16,25,26	15,16,27,28	15,16,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28					
	ROW 3	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____					
	ROW 4	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34					
PRESTRESSING FORCE IMMEDIATELY AFTER STRAND RELEASE, P _{pt} , (KIPS/BEAM)		327	328	328	391	391	453	454	454	515	516	517					
EFFECTIVE PRESTRESSING FORCE AFTER ALL LOSSES, P _{pe} , (KIPS/BEAM)		297	298	298	350	352	403	404	406	454	456	458					
REQUIRED FACTORED MOMENT @ STRENGTH I, M _u (FT-KIPS/BEAM)		359	393	427	465	504	545	588	639	693	746	800					
FACTORED FLEXURAL RESISTANCE, M _r (FT-KIPS/BEAM)		527	527	527	644	644	743	743	743	853	853	853					
TOTAL NO. DEBONDED STRANDS		_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____					
DEBONDED STRAND POSITION NUMBER & SHIELDING LENGTH FROM EACH END	ROW 1	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____					
	ROW 2	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____	_____					
NUMBER & LENGTH *4 ET TOP TENSION BARS @ EACH END		3 - *4 x 4' -6"	3 - *4 x 4' -6"	3 - *4 x 5' -0"	3 - *4 x 5' -0"	3 - *4 x5' -0"	3 - *4 x 5' -6"	3 - *4 x 5' -6"	3 - *4 x 6' -0"	3 - *4 x 6' -0"	3 - *4 x 6' -0"	3 - *4 x 6' -6"					
NUMBER & LENGTH *5 BT BOTTOM TENSION BARS @ EACH END		2 - *6 x 5' -0"	2 - *5 x 5' -0"	2 - *6 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"	2 - *5 x 6' -0"					
DESIGN CAMBER + = POSITIVE (UP) (INCHES)	@ RELEASE	0.15	0.15	0.15	0.26	0.27	0.37	0.38	0.38	0.55	0.55	0.54					
	@ ERECTION	0.23	0.23	0.22	0.40	0.39	0.56	0.55	0.52	0.79	0.76	0.71					
	@ FINAL	0.24	0.22	0.18	0.40	0.36	0.55	0.49	0.40	0.70	0.59	0.45					
NUMBER & SPACING OF TL-2 GUARDRAIL INSERTS SEE NOTE 6	NO OF INSERTS REQD.																
	END OF BEAM TO ϕ OF FIRST INSERT E.A. END																
	ϕ OF 1st INSERT TO ϕ 2nd INSERT E.A. END																
WEIGHT OF TYPICAL BEAM INCLUDING DIAPHRAGM (TONS)		9.2	9.8	10.4	10.9	11.5	12.0	12.6	13.1	13.8	14.3	14.9					

NOTES

1. BEAM WEIGHTS LISTED IN THE DESIGN TABLE ARE BASED ON ZERO SKEW, 2 FT. LONG ENDBLOCK AND DIAPHRAGMS SPACED @ 15 FT C/C. WEIGHTS FOR SKEWED BEAMS, LONGER ENDBLOCKS AND ADDITIONAL DIAPHRAGMS SHOULD BE ADJUSTED ACCORDINGLY.

FOR ADDITIONAL DIAPHRAGMS, ADD 226 LBS/DIAPHRAGM.
FOR SKEW ADD 21 LBS/DEGREE OF SKEW/END.
FOR LONGER ENDBLOCK, ADD 271 LBS/LF/END.
2. DESIGNERS SHOULD NOTE THAT DATA IN STANDARD TABLE IS BASED ON EVEN SPAN LENGTHS, A TWO LANE STRUCTURE 8 BEAMS WIDE AND ZERO SKEW. SUPERIMPOSED DEAD LOADS INCLUDE TYPE F PARAPET (321 PLF) AND A FWS OF 50 PSF. FOR NON-STANDARD BRIDGES DATA SHOULD BE VERIFIED AND IF REQUIRED NEW DESIGN DATA ENTERED INTO BLANK COLUMNS. IN NO CASE SHALL THE STANDARD DESIGN TABLE BE ALTERED.
3. IF BEAM DOES NOT MEET ALL TOLERANCES REFER TO MP 603.10.40 FOR GUIDANCE. MEASUREMENT OF CAMBER FOR COMPARISON TO PREDICTED DESIGN VALUES SHOULD BE COMPLETED WITHIN 72 HOURS OF RELEASE. ADDITIONALLY, CAMBER SHOULD BE EVALUATED UNDER CONDITIONS THAT MINIMIZE THE EFFECT OF TEMPERATURE VARIATION.

4. DESIGNER, FABRICATOR, AND ERECTOR SHALL BE AWARE THAT SKEWED END BEAMS MAY TWIST OR WARP, CAUSING UNEVEN BEAM SEATING AT THE BEARINGS. THE CONTRACTOR IS REQUIRED TO CORRECT AT THE TIME OF ERECTION,. BEFORE THE BEAMS ARE SECURED IN PLACE. METHOD OF CORRECTION SHALL PROVIDE AN EVEN, TOTAL BEARING AND A LEVEL TOP BEAM SURFACE. TOLERANCE, AFTER CORRECTION, SHALL BE (+/-) $\frac{1}{8}$ INCH. THE FABRICATOR SHALL NOTIFY THE CONTRACTOR AND DESIGNER IF CORRECTIONS ARE REQUIRED PRIOR TO SHIPMENT.
5. MAXIMUM BEAM SKEW SHALL BE 30 DEGREES.
6. DESIGNER INPUT VALUES OF NUMBER OF INSERTS, DISTANCE FROM END OF BEAM TO ϕ FIRST INSERT, AND ϕ FIRST INSERT TO ϕ SECOND INSERT. ABOVE VALUES SHALL BE BASED ON THE REQUIRED 6'-3" GUARDRAIL POST SPACING ACROSS THE BRIDGE.
7. THIS SHEET SHALL BE USED IN CONJUNCTION WITH STANDARD SHEETS BR-B21A, BR-B100, BR-B101, BR-B102A & B, BR-B103, BR-B104, BR-B105A & B AND BR-B106 AS APPLICABLE.

APPROVED: _____
DIRECTOR, ENGINEERING DIVISION

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
ENGINEERING DIVISION

DESIGN TABLE FOR 21"

PRESTRESSED BOX BEAM

REVISED STANDARD SHEET BR-B21B

PREPARED:
07-02-07

REVISD:
07-10 TW

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
ENGINEERING DIVISION

DESIGNED BY:TW/
DRAWN BY:THB/
CHECKED BY:TW/
REVIEWED BY:THB/
DATE:
SCALE:
SHEET NO OF
BRIDGE NUMBER

DESIGN TABLE FOR 21"

PRESTRESSED BOX BEAM