

DESIGN DATA FOR 27" DEPTH ADJACENT BOX BEAM

SPAN LENGTH ϕ TO ϕ BEARING		40'-0"	42'-0"	44'-0"	46'-0"	48'-0"	50'-0"	52'-0"	54'-0"	56'-0"	58'-0"	60'-0"				
OVERALL LENGTH OF BEAM		41'-6"	43'-6"	45'-6"	47'-6"	49'-6"	51'-6"	53'-6"	55'-6"	57'-6"	59'-6"	61'-6"				
NO. OF 270 KSI, $\frac{1}{2}$ " ϕ LOW-RELAXATION STRANDS, AREA/STRAND = 0.167 SQ. IN.		10	10	12	12	12	12	14	14	16	16	18				
STRAND POSITION NUMBER	ROW 1	1,2,11,12	1,2,11,12	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,7,8,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14	1,2,5,6,9,10,13,14				
	ROW 2	15,16,25,26	15,16,25,26	15,16,27,28	15,16,27,28	15,16,27,28	15,16,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,21,22,27,28	15,16,19,20,23,24,27,28				
	ROW 3	————	————	————	————	————	————	————	————	————	————	————				
	ROW 4	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34	33,34				
PRESTRESSING FORCE IMMEDIATELY AFTER STRAND RELEASE, P_{pi} , (KIPS/BEAM)		329	329	392	393	393	393	456	457	519	519	581				
EFFECTIVE PRESTRESSING FORCE AFTER ALL LOSSES, P_{pe} , (KIPS/BEAM)		301	302	355	356	357	358	411	413	463	465	514				
REQUIRED FACTORED MOMENT @ STRENGTH I, M_u (FT-KIPS/BEAM)		563	608	660	717	772	829	887	946	1007	1069	1132				
FACTORED FLEXURAL RESISTANCE, M_r (FT-KIPS/BEAM)		706	706	868	868	868	868	1011	1011	1164	1164	1299				
TOTAL NO. DEBONDED STRANDS		————	————	————	————	————	————	————	————	————	————	————				
DEBONDED STRAND POSITION NUMBER & SHIELDING LENGTH FROM EACH END	ROW 1	————	————	————	————	————	————	————	————	————	————	————				
	ROW 2	————	————	————	————	————	————	————	————	————	————	————				
NUMBER & LENGTH *4 ET TOP TENSION BARS @ EACH END		3 - *4 x 5'-6"	3 - *4 x 5'-6"	3 - *4 x 6'-0"	3 - *4 x 6'-0"	3 - *4 x 6'-0"	3 - *4 x 6'-6"	3 - *4 x 6'-6"	3 - *4 x 7'-0"	3 - *4 x 7'-0"	3 - *4 x 7'-0"	3 - *4 x 7'-6"				
NUMBER & LENGTH *5 BT BOTTOM TENSION BARS @ EACH END		6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-0"	6 - *5 x 7'-6"	4 - *5 x 7'-6"	4 - *5 x 7'-6"	4 - *5 x 8'-0"				
DESIGN CAMBER + = POSITIVE (UP) (INCHES)	@ RELEASE	0.13	0.13	0.23	0.23	0.22	0.21	0.33	0.32	0.48	0.47	0.62				
	@ ERECTION	0.18	0.15	0.34	0.31	0.28	0.24	0.42	0.37	0.64	0.58	0.82				
	@ FINAL	0.13	0.07	0.14	0.22	0.14	0.04	0.25	0.13	0.44	0.30	0.55				
NUMBER & SPACING OF TL-2 GUARDRAIL INSERTS SEE NOTE 6	NO OF INSERTS REQD.															
	END OF BEAM TO ϕ OF FIRST INSERT EA. END															
	ϕ OF 1st INSERT TO ϕ 2nd INSERT EA. END															
WEIGHT OF TYPICAL BEAM INCLUDING DIAPHRAGM (TONS)		13.8	14.4	15.0	15.7	16.3	16.9	17.5	18.1	18.7	19.3	19.9				

NOTES

1. BEAM WEIGHTS LISTED IN THE DESIGN TABLE ARE BASED ON ZERO SKEW, 2 FT. LONG ENDBLOCK AND DIAPHRAGMS SPACED @ 15 FT C/C. WEIGHTS FOR SKEWED BEAMS, LONGER ENDBLOCKS AND ADDITIONAL DIAPHRAGMS SHOULD BE ADJUSTED ACCORDINGLY.

FOR ADDITIONAL DIAPHRAGMS, ADD 361 LBS/DIAPHRAGM.
FOR SKEW ADD 27 LBS/DEGREE OF SKEW/END.
FOR LONGER ENDBLOCK, ADD 433 LBS/LF/END.
2. DESIGNERS SHOULD NOTE THAT DATA IN STANDARD TABLE IS BASED ON EVEN SPAN LENGTHS, A TWO LANE STRUCTURE 8 BEAMS WIDE AND ZERO SKEW. SUPERIMPOSED DEAD LOADS INCLUDE TYPE F PARAPET (321 PLF) AND A FWS OF 50 PSF. FOR NON-STANDARD BRIDGES DATA SHOULD BE VERIFIED AND IF REQUIRED NEW DESIGN DATA ENTERED INTO BLANK COLUMNS. IN NO CASE SHALL THE STANDARD DESIGN TABLE BE ALTERED.
3. IF BEAM DOES NOT MEET ALL TOLERANCES REFER TO MP 603.10.40 FOR GUIDANCE. MEASUREMENT OF CAMBER FOR COMPARISON TO PREDICTED DESIGN VALUES SHOULD BE COMPLETED WITHIN 72 HOURS OF RELEASE. ADDITIONALLY, CAMBER SHOULD BE EVALUATED UNDER CONDITIONS THAT MINIMIZE THE EFFECT OF TEMPERATURE VARIATION.

4. DESIGNER, FABRICATOR, AND ERECTOR SHALL BE AWARE THAT SKEWED END BEAMS MAY TWIST OR WARP, CAUSING UNEVEN BEAM SEATING AT THE BEARINGS. THE CONTRACTOR IS REQUIRED TO CORRECT AT THE TIME OF ERECTION., BEFORE THE BEAMS ARE SECURED IN PLACE. METHOD OF CORRECTION SHALL PROVIDE AN EVEN, TOTAL BEARING AND A LEVEL TOP BEAM SURFACE. TOLERANCE, AFTER CORRECTION, SHALL BE (+/-) $\frac{1}{8}$ INCH. THE FABRICATOR SHALL NOTIFY THE CONTRACTOR AND DESIGNER IF CORRECTIONS ARE REQUIRED PRIOR TO SHIPMENT.
5. MAXIMUM BEAM SKEW SHALL BE 30 DEGREES.
6. DESIGNER INPUT VALUES OF NUMBER OF INSERTS, DISTANCE FROM END OF BEAM TO ϕ FIRST INSERT, AND ϕ FIRST INSERT TO ϕ SECOND INSERT. ABOVE VALUES SHALL BE BASED ON THE REQUIRED 6'-3" GUARDRAIL POST SPACING ACROSS THE BRIDGE.
7. THIS SHEET SHALL BE USED IN CONJUNCTION WITH STANDARD SHEETS BR-B27A, BR-B100, BR-B101, BR-B102A & B, BR-B103, BR-B104, BR-B105A & B AND BR-B106 AS APPLICABLE.

APPROVED: ----- DIRECTOR, ENGINEERING DIVISION ----- DATE:-----	
WEST VIRGINIA DEPARTMENT OF TRANSPORTATION DIVISION OF HIGHWAYS ENGINEERING DIVISION	
DESIGN TABLE FOR 27"	
PRESTRESSED BOX BEAM	
REVISED STANDARD SHEET BR-B27B	
PREPARED: 07-02-07	
REVISED:	
07-10	TW

STATE PROJECT NUMBER	FEDERAL PROJECT NUMBER	STATE DIST. NO.	COUNTY	SHEET NO.	TOTAL SHEETS

MIN. CONCRETE STRENGTH @ RELEASE = 5500 PSI
MIN. CONCRETE STRENGTH @ 28 DAYS = 8000 PSI
INITIAL PULL/STRAND = 33,820 LBS
CROSS-SECTION AREA/STRAND = 0.167 SQ.IN.

WEST VIRGINIA DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
ENGINEERING DIVISION

	DESIGNED BY:THB/
	DRAWN BY:THB/
	CHECKED BY:TM/
	REVIEWED BY:TW/
	DATE:
DESIGN TABLE FOR 27" PRESTRESSED BOX BEAM	SCALE:
	SHEET NO OF
	BRIDGE NUMBER